

#### TERRY E. BRANSTAD, GOVERNOR KIM REYNOLDS, LT. GOVERNOR

### STATE OF IOWA

DEPARTMENT OF NATURAL RESOURCES CHUCK GIPP, DIRECTOR

August 20, 2015

Mr. William Gibson, Environmental Manager Roquette America, Inc. P.O. Box 6647 1003 South Fifth Street Keokuk, IA 52632

RE: Roquette America, Inc. (RAI) Wastewater Treatment Facility Improvement project-DNR Project Number S2013--0183

Subject: Variance Request from 567 IAC 64.2(9)c and Design Standards Section 18B 4.2

Dear Mr.Gibson:

After careful and thorough consideration, the Department has <u>approved</u> your June 30, 2015 request for a variance from Iowa Administrative Code Subrule 567 IAC 64.2(9)c and Chapter 18B 4.2 of the Iowa Wastewater Facilities Design Standards, which requires the maximum allowed aeration tank organic loading is 40 lbs BOD<sub>5</sub>/1,000 cu ft/day, the maximum allowed F/M ratio is 0.5 lb BOD<sub>5</sub>/lb of MLVSS and the maximum allowed MLSS is 3000 mg/l.

Based on the documentation presented by your Engineer, it is the determination of this Department that satisfactory justification has been presented to warrant the granting of a variance for the organic loading 52 lbs/day BOD<sub>5</sub>/1,000 cu ft, 0.22 lbs BOD<sub>5</sub>/day/lb of MLVSS and 6,400 mg/l MLSS. The requested variance is deemed to be reasonable and necessary pursuant to the Iowa Code section 455B.181.

The facts presented for the project present unique circumstances and the variance is therefore justified to provide the narrowest exception possible to the provisions of the rule in accordance with Rule 561 IAC 10.5

This decision is based on our review of justification presented to support the request. Our concurrence with the request is based on the Department's finding that the resulting project will provide substantially equivalent effectiveness as would be provided by technical compliance with the design standard on this issue. Please feel free to contact Suresh Kumar at (515) 725-8429 or email <u>suresh.kumar@dnr.lowa.gov</u> if you have any questions.

Sincerely,

J.C.M.C

Jon Tack Water Quality Bureau Chief

cc: Huff & Huff Engineers DNR FO 6 DNR Sewage File # 6-56-40-101

		VARIANCE REQ	UEST	
		Iowa Department of Natu	ral Resou	urces
1.	Date:	08/20/15	14a.	Decision: Date HPDd.
2.	Reviewer/Engr.:	Suresh Kumar		Date: 5/25/15 Expiration Date
3.	Date Received:	06/30/15	14b.	(if any): Permanant
4.	Facility Name:	Roquette America, Inc. (RAI)		
5.	Facility Number:	6-56-40101		
6.	County Number:	56, Lee	15.	Appealed:
7.	Program Area:	CP (Wastewater)		Date:
8.	Facility Type:	C O5		
		332, Activated Sludge Organic		
9.	Subject Area:	Loading Rates		
10.	Rule Reference:	567-64.2(9)a		
11.	Design Std. Ref.:	18B.4.2		
12.	Consulting Engr.:	Huff & Huff Engineers		
13.	Variance Rule:	Variance Rule: 567-64.2(9)c		

16. Description of Variance Request:

Description of Variance Request: RAI is a corn wet milling industrial facility located in Keokuk, Iowa. RAI has an activated sludge wastewater treatment system that treats their wastewater that has been upgraded in 2012 with two oxidation ditch aeration basins. RAI is anticipating production increase at their industrial operation that will increase the organic and hydraulic waste loads to their WWTP. Therefore, RAI is requesting a variance from the Design Standard 18B.4.2 to allow higher organic loadings to the aeration tanks instead of the maximum allowed 40 lbsBOD5/day/1,000 cu ft. The higher organic loading increases the food to micro-organism ratio (MLVSS) and mixed liquor suspended solids (MLSS) ratio more than the design standards.

The new aeration system, oxidation ditch, at RAI is a complete mix reactor activated sludge system with inplace anoxic zones. Per the Design Standard Chapter 18B.4.2 the maximum allowed aeration tank organic loading is 40 lbs BOD5/1,000 cu ft/day, the maximum allowed F/M ratio is 0.5 lb BOD5/lb MLVSS and the maximum allowed MLSS is 3000 mg/l. The requested variance is for 52 lbs/day/1,000 cu ft, 0.22 lbs BOD5/day/lb of MLVSS and 6,400 mg/l MLSS.

#### 17. Applicant's/Consulting Engineer's Justification:

2012 WWTP upgrade RAI selected oxidation ditch for the treatment of high strength organic waste. At the time IDNR developed the Design Standards Oxidation ditches were not popular and therefore, the high strength waste reduction utilizing oxidation ditches were not addressed in the design specifications. The organic waste generated at RAI is readily degradable high strength industrial waste. Previously permitted activated sludge WWTP at RAI had higher organic loading, 72 lbs BOD5/day 1,000 cu ft, compared to the requested loading for the expanded new facility, 52 lbs BOD5/day/ 1,000 cu ft. Literature search provided along with the variance request and the RAI WWTP performance data analysis has proven that the current treatment system is capable of consistently achieving the effluent limits.

18. Department's Justification:

Recommend this variance request to be approved.

18B.4.2 capacities and permissible loadings represent typical domestic wastewater. Design standard allows aeration tank sizing based on full scale plant operation experience and rational calculations. The initial variance request was submitted to the department on June 14, 2014 along with the revised facility plan. Department conducted an initial review of the request and suggested RAI to submit a detailed WWTP operational data including plant influent loadings and effluent data. On June 30, 2015 department received detailed field validated data for the WWTP influent loadings and effluent data. The data analysis has proved that the oxidation ditches have performed successfully and achieved effluent limits consistently. The data shows that the WWTP influent ammonia has averaged 128 mg/l with the peak month 225 mg/l and the effluent concentration averaged at 2.2 mg/l and peak month at 8.9 mg/l. The effluent BOD5 averaged at 2,113 lbs/day compared to the permit effluent limit of 5,924 lbs/day. The TSS effluent averaged 1,386

lbs/day compared to the effluent permit limit of 7,138 lbs/day.

The consulting engineer has conducted detailed literature search and provided the information to the department along with the revised variance request submittal. The literature search includes- Industrial Water Pollution Control Book by Eckenfelder published on 1996, Biological Process Design for Wastewater Treatment by Benfield and Randall dated 1980, 1978 USEPA publication on Biological Treatment of Wastes from the Wet Corn Milling Industry, and Biological Wastewater Treatment by Grady, C. P. published on 1999. These publications emphases that well operated oxidation ditches are capable of reducing high strength organic loads in the WWTP.

Therefore, from the data analysis and literature review the department believes that the improved new WWTP with the oxidation ditches at RAI WWTP could treat the proposed higher organic loadings and meet effluent limits consistently.

19. Precedents Used:	
None	
20. Staff Reviewer: Smearthann	Date: 082515
21. Supervisor: Jalyp Chennepati	Date: 8/25/15
22. Authorized by: A Cac	Date: 8/25/15
	/ /



915 Harger Road, Suite 330 Oak Brook, IL 60523 Phone (630) 684-9100 Fax (630) 684-9120 Website: http://huffnhuff.com

June 25, 2015

Mr. Suresh Kumar Wastewater Treatment Review Engineer; Industrial Iowa Department of Natural Resources 502 East 9<sup>th</sup> Street Des Moines, IA 50319

Re: Roquette America, Inc. Performance Summary

Dear Mr. Kumar:

On June 14, 2014, Roquette America, Inc. (Roquette) submitted a Revised Facility Plan to the Iowa DNR. This was followed up with a variance request submitted on August 12, 2014 and supplemented on August 25, 2014. Variances were requested from three specific design requirements contained in Chapter 18B, Activated Sludge Biological Treatment. Specifically, relief from the following were requested:

- 1. Maximum Aeration Tank Organic Loading in pounds BOD<sub>5</sub> per day per 1,000 cu ft
- 2. Food to Micro-organism ratio in pounds BOD<sub>5</sub> per pound MLVSS per day
- 3. Mixed Liquor Suspended Solids (MLSS) operating range concentration

The justifications for these requests were described in detail in both the Revised Facility Plan and the variance request. The purpose of this letter is to summarize performance since the new oxidation ditches came on line.

The three technical design parameters from which Roquette is seeking a variance are all related to the two oxidation ditches/clarifiers installed as part of the expansion. As the DNR is aware, oxidation ditches were not common when the design standards were developed, nor were highstrength wastes included in the development of the design specifications. Oxidation ditches are a form of activated sludge that has become popular over the past decade because of the stability achieved, plus the ability to incorporate denitrification in the process.



With the higher organic-strength waste associated with Roquette's operation, oxidation ditches were selected, have been constructed, and placed into operation over the first half of 2014. The oxidation ditches are designed to achieve the following:

- 1) Reduce BOD<sub>5</sub>
- 2) Reduce Total Suspended Solids (TSS)
- 3) Oxidize ammonia to nitrates
- 4) Denitrify the nitrates to nitrogen gas

In addition, the oxidation ditches provided for a significant increase in both organic loading and hydraulic loading capacity to address both existing loadings and future production growth at the facility. According to the Facility Plan, the expanded treatment plant is designed for the following:

	Flow, mgd	BOD5, lbs/day	NH <sub>3</sub> /N, lbs/day	TSS, lbs,day
AWW	4.89	74,526	5,659	15,482
MWW	7.34	110,533	8,807	74,296

To address the question raised by the Iowa DNR regarding performance of the oxidation ditches, a review of the treatment plant performance for the above parameters, plus effluent nitrates is appropriate. Table 1 presents the influent flow, BOD<sub>5</sub>, and TSS loading since 2011. Monthly flows have averaged 4.06 mgd and the maximum monthly average flow was 4.67 mgd. The monthly BOD<sub>5</sub> loading has averaged 50,020 pounds per day with a monthly maximum of 90,385 pounds per day. The TSS loadings have averaged 7,741 pounds per day, and the maximum monthly average loading was 15,901 pounds per day. From these loadings, it is clear that the expanded treatment plant facilities have capacity for future production increases. The influent ammonia concentrations since January 2011 to the treatment plant are presented in Table 2. The influent ammonia has averaged 128 mg/L with the peak month 225 mg/L.

Of course loadings don't equate to performance. Table 3 presents the effluent  $BOD_5$  performance since the oxidation ditches were brought on line. The  $BOD_5$  removal across the expanded treatment plant has been excellent, with the highest 30-day average  $BOD_5$  only 2,113 pounds per day, compared to the permit limit of 5,924 pounds per day. Table 4 presents the TSS effluent results. On a monthly average, the system has averaged only 1,386 pounds per day, compared to the NPDES monthly limit of 7,138 pounds per day. The maximum month discharge was 5,576 pounds per day, still well below the monthly permit limit. Table 5 presents the effluent ammonia results on a monthly average basis. The effluent concentration has averaged 2.2 mg/L, with a maximum monthly average of 8.9 mg/L. Consistent nitrification has been achieved. Finally, Table 6 presents the effluent nitrate results since January 2014, when the

oxidation ditches were started up. Effluent nitrate concentration (as N) has averaged a low 4.0 mg/L since start-up, very impressive compared to the influent ammonia concentration, which has averaged above 100 mg/L.

Based on the incremental MWW BOD<sub>5</sub> loading of 19,593 lbs per day based on the requested design loading to the existing permitted loading, and 5,000,000 gallons of oxidation ditches of aeration capacity, the MWW loading will be 29 lbs per day per 1,000 cu ft, compared to the MWW design of the originally permitted system of 118 lbs BOD<sub>5</sub> per day per 1,000 cu ft.

If the entire aeration system is aggregated, the total  $BOD_5$  loadings are 74,526 lbs per day AWW and 110,533 lbs per day at the MWW. The total aeration capacity available is 10,760,000 gallons. The overall organic loading compared to the existing system is presented below:

Conditions			AWW BOD <sub>5</sub> Loading,	MWW BOD <sub>5</sub> Loading,	
			lbs/day/1,000 cu ft	lbs/day/1,000 cu ft	
Previously Permitted			72	118	
Requested	for	Expanded	52	77	
Facilities					

Roquette's oxidation ditches are designed for an MLVSS of 6,400 to 8,000 mg/L, or an active biomass (MLVSS) in five million gallons of 334,000 pounds of MLVSS. At the design organic loading 74,526 pounds of BOD<sub>5</sub> per day, and ignoring any BOD<sub>5</sub> reduction before the oxidation ditches, the loading on the ditch will be **0.22** pounds BOD<sub>5</sub> per day per pound of MLVSS.

The existing system has demonstrated the ability to achieve the effluent  $BOD_5$  and ammonia effluent limits at the current loadings. The oxidation ditches essentially doubles the aeration capacity and retention time, with the expectation that the expanded system will prove more reliable than the current system. The higher loadings are common for the food industry, which has more readily degradable substrate than domestic sewage. So long as sufficient oxygen is provided, which is the case here, the lower organic loadings will improve performance and reliability.

It should be noted that the BOD<sub>5</sub> loading factor is not used in design of activated sludge systems. The Iowa design standards at Chapter 18B includes a table of "Typical Aeration Tank Loadings and Design Parameters" which indicates the maximum organic loading for Extended Aeration should be 15 pounds of BOD<sub>5</sub> per 1,000 cu ft of aeration capacity per day. There is no theoretical basis for this limit, and it is based on data from domestic sewage, with an influent BOD<sub>5</sub> of less than 200 mg/L. This same Iowa table indicates the Food/Micro-organism ratio (F/M) should be in the range of 0.05 to 0.1 pounds of BOD<sub>5</sub> per pound MLVSS per day, and the MLSS should be

in the range of 3,000 to 5,000 mg/L. None of these guidelines are applicable for high strength, readily degradable wastes. Going back to 1966, Eckenfelder published his <u>Industrial Water</u> <u>Pollution Control</u> book that includes some industrial loadings on a volumetric basis, and more on a MLSS basis. The volumetric design loadings ranged from19 to 475 pounds BOD<sub>5</sub> per day per 1,000 cu ft, all above the Iowa guidelines. In 1978 at the Purdue Industrial Waste Conference, Barrett and Hayden reported on a Brewery activated sludge facility operating at an average 23.8 pounds of BOD<sub>5</sub> per day per 1,000 cu ft and an F/M ratio of 0.15 per day (page 865). Biesinger, et al. at the Purdue Industrial Waste Conference reported that for a Brewery wastewater organic loadings ranged from 15 to 1,180 pounds BOD<sub>5</sub> per day per 1,000 cu ft. (35<sup>th</sup> Conference proceedings at page 600, 1980.) On a mass basis, the organic loadings range from a low of 0.14 to 3.0 pounds of BOD<sub>5</sub> per day per pound of MLSS. To adjust to per pound of MLVSS, these values would be adjusted upwards by approximately 25 percent. Again, all of them are higher than the upper range in the Iowa DNR "Typical" Loadings.

The reason for including this reference material is to note the inapplicability of municipal loadings for industrial wastewater. In Benefield and Randall's book, <u>Biological Process Design</u> for Wastewater Treatment, (1980), loadings for high rate activated sludge are reported to range from 1.5 to 5.0 pounds BOD<sub>5</sub> per day per pound MLVSS (page 187), over an order of magnitude higher than the Iowa *typical* loadings for municipal wastewater. Koon and Adams reported at the 30<sup>th</sup> Purdue Industrial Waste Conference on an organic chemical manufacturer operating between 0.23 and 0.43 pounds BOD<sub>5</sub> per day per pound MLVSS, with removal efficiencies ranging from 94.4 and 97.8 percent (Page 911).

In 1978, USEPA published *Biological Treatment of Wastes from the Wet Corn Milling Industry*. This report describes the pilot- and full-scale results for a new activated sludge process, sized for 18,800 pounds per day COD and an aeration capacity of 650,000 gallons. This equates to a COD loading of 216 pounds of COD per day per 1,000 cu ft. Assuming a 1.5:1 ratio of COD to BOD<sub>5</sub>, this equates to a BOD<sub>5</sub> loading of 144 pounds per day BOD<sub>5</sub> per 1,000 cu ft, compared to the *typical* Iowa guidelines of 15 pounds of BOD<sub>5</sub> per day per 1,000 cu ft. This wet corn milling facility was sized for 1.16 pounds of COD per day per pound MLSS, or using the same 1.5 ratio, 0.78 pounds of BOD<sub>5</sub> per day per pound of MLSS, or **0.97** pounds of BOD<sub>5</sub> per day per pound of MLVSS. This value is considerably higher than the Iowa *typical* extended aeration loading range of **0.05 to 0.10** pounds per day per pound MLVSS for municipal wastewater.

The point of the above is merely to point out that the Iowa design guidance for activated sludge systems is not appropriate for readily degradable high-strength industrial wastewater. Eckenfelder's long-recognized equation for complete mix aeration tank is simply:

$$dS/dt = \underline{K' X S_e}$$

Where:

dS/dt is the substrate utilization rate (mg/L/day) K' is Eckenfelder's modified model substrate removal rate constant (mg/mg/day) S<sub>e</sub> is the effluent substrate concentration (mg/L) S<sub>i</sub> is the influent substrate concentration (mg/L) X is the MLVSS concentration (mg/L)

Looking at this equation, the **rate of substrate utilization** (dS/dt) is really the key. If the waste is readily biodegradable, a high degree of removal is possible. In the Iowa guidance, this factor is not considered, resulting in a *one size fits all* approved for design of activated sludge systems, which the above examples clearly show is not how such systems are designed.

The definitive book on designing biological wastewater systems is Grady, C. P., et al, <u>Biological</u> <u>Wastewater Treatment</u> (1999). Grady explains as follows:

Activated sludge is applicable to wastewaters with a wide range of concentrations....aerobic suspended growth bioreactors are typically used to treat wastewaters with biodegradable chemical oxygen demand (COD) concentrations up to 4,000 mg/L....Technically, activated sludge can be used to treat wastewaters with biodegradable COD concentrations ranging from about 50 to 10,000 mg/L. The technical constraints derive from sludge settleability and the ability to maintain a suitable Solids Retention Time (SRT).

For high-concentration wastewaters, the constraint relates to the ability of the clarifier to consolidate the MLSS for recycle to the bioreactor (page 387).

Grady goes on then to explain how to design a complete mix activated sludge system, starting on page 419. Nowhere in the design process does the loading per aeration volume enter into the design calculations.

In summary, the expansion with the oxidation ditches has performed successfully, achieving effluent limits on a consistent basis, and overall demonstrating its ability to handle spikes in loadings and flow. Like all biological processes, the oxidation ditch system can be upset from unplanned discharges in the production area, but this would be true no matter what loadings the system was designed to handle. The data presented herein support Roquette's request for a variance, and can be used by the IDNR as evidence that the wastewater treatment system can handle the requested design loadings and consistently achieve its effluent limits.

#### Signed release:



Please contact William Gibson (319-526-3411) or Huff & Huff, Inc. at 630-684-4444 with any questions.

Sincerely;

Lude I Huff

Linda L. Huff, P.E. Vice President

William J. Gibson Environmental Department Manager

#### Influent Flow, MGD BOD5 Loading, lb/day TSS Loading, lb/day Month Mean Daily Max Mean Daily Max Mean Daily Max 4.07 4.54 49,103 78,050 8.062 28,089 Jan-11 Feb-11 3.82 4.40 44,913 73,913 8,950 31,375 Mar-11 35,240 3.93 4.41 56,318 75,373 6,542 Apr-11 3.94 4.39 63,526 82,928 8,121 21,629 May-11 3.83 4.36 57,634 90,172 12,130 40,710 Jun-11 81,869 9,372 27,259 3.69 4.22 50,821 Jul-11 3.83 4.50 53,474 76,240 7,948 22,308 48,566 Aug-11 3.68 4.41 45,826 72,526 12,482 Sep-11 3.99 4.32 61,586 86,251 10,739 21,578 Oct-11 3.92 4.41 48,519 99,747 7,518 12,233 Nov-11 41,523 11,290 3.58 4.24 61,912 6,288 4.47 74,047 Dec-11 4.00 51,774 6,897 11,776 Jan-12 3.95 4.40 57,915 97.038 7.926 12,768 Feb-12 4.04 4.62 52,548 105,865 4,997 18,587 Mar-12 3.23 4.26 48.556 192,154 2.625 5,329 Apr-12 3.89 4.51 44,129 61,226 5,146 20,912 May-12 4.42 68,503 3.83 44,748 4,277 18,370 Jun-12 3.87 4.39 44,014 57,452 4,322 10,551 Jul-12 44,649 3.85 4.46 72,495 5,373 18,195 Aug-12 3.95 4.40 45,264 78,936 5,709 20,387 Sep-12 3.93 4.46 37,812 60,598 5,799 23,424 Oct-12 3.60 4.38 36,055 68,573 5,505 17,113 Nov-12 4.07 4.57 45,076 64,099 5,438 14,091 Dec-12 4.00 4.87 37,701 60,143 4,486 8,669 Jan-13 3.96 4.47 40,063 66,060 7,244 17,154 Feb-13 4.04 4.64 43,276 67,035 8,410 16,904 Mar-13 4.73 4.16 47,731 68,685 10,369 21,729 Apr-13 4.15 5.04 46,712 64,880 13,086 46,269 May-13 4.06 4.91 41,552 63,933 30,408 9,635 Jun-13 3.95 4.53 49,193 86,105 10.881 30,434 Jul-13 4.16 4.77 40,257 53,669 10,101 32,358 Aug-13 4.14 4.90 45,688 97,444 10,277 33,841 Sep-13 3.86 4.76 42,208 92,331 6,198 19,655 Oct-13 4.37 4.71 46,381 61,860 6,518 15,744 Nov-13 3.79 4.45 37,276 87,401 4.404 10,495 Dec-13 3.79 4.40 37,029 55,475 5,257 20,333 Jan-14 4.01 4.70 43,083 74,391 4,749 7,662 Feb-14 3.95 4.85 67,024 167,905 5,590 14,028 Mar-14 4.27 5.17 57,318 91,508 5,636 9,944 Apr-14 4.37 4.87 59.347 88,679 7,667 16,083 May-14 4.38 4.93 90,385 131,153 7,429 14,459 Jun-14 4.49 5.44 53,171 72,780 7,466 16,817

49,450

90,385

7,839

55,002

## TABLE 1 ROQUETTE WASTEWATER TREATMENT PLANT INFLUENT LOADINGS

4.97

4.22

Jul-14

## TABLE 1 ROQUETTE WASTEWATER TREATMENT PLANT INFLUENT LOADINGS

Month Influent Flow, MGD		Flow, MGD	BOD5 Loading, lb/day		TSS Load	TSS Loading, lb/day	
	Mean	Daily Max	Mean	Daily Max	Mean	Daily Max	
Aug-14	4.67	5.19	44,155	69,757	11,907	71,916	
Sep-14	4.31	4.88	49,904	79,614	9,986	68,917	
Oct-14	4.63	9.25	56,140	117,546	6,595	27,851	
Nov-14	4.09	5.15	49,451	73,571	7,291	14,404	
Dec-14	4.51	4.93	55,728	71,703	6,355	16,234	
Jan-15	4.54	5.61	53,481	74,367	6,709	13,095	
Feb-15	4.14	4.92	50,567	80,598	9,382	98,432	
Mar-15	4.56	5.15	66,220	126,687	8,026	29,052	
Apr-15	4.64	5.07	70,432	276,472	15,901	54,597	
May-15	4.56	4.95	54,371	69,777	12,699	56,828	
Mean	4.06	4.77	50,020	86,073	7,741	26,058	
Maximum	4.67	9.25	90,385	276,472	15,901	98,432	

Month	Average NH3/N,	Daily Max NH3/N,
	mg/L	mg/L
11-Jan	134	292
11-Jah 11-Feb	134	292
11-Mar	140	213
11-Apr	140	262
11-May	134	232
11-Jun	135	204
11-Jul	133	314
11-Aug	182	361
11-Sep	223	346
11-Oct	198	518
11-Nov	154	211
11-Dec	184	229
12-Jan	171	249
12-Feb	138	245
12-Mar	98	192
12-Apr	160	421
12-May	159	253
12-Jun	142	200
12-Jul	174	401
12-Aug	157	232
12-Sep	151	262
12-Oct	124	246
12-Nov	138	209
12-Dec	107	188
13-Jan	115	195
13-Feb	119	232
13-Mar	119	188
13-Apr	142	230
13-May	129	200
13-Jun	126	185
13-Jul	135	314
13-Aug	99	174
13-Sep	108	278
13-Oct	115	174
13-Nov	107	167
13-Dec	101	190
14-Jan 14 Eeb	94	137
14-Feb 14-Mar	112	184
14-Mar 14-Apr	92 81	153
14-Apr 14-May	57	175
14-May 14-Jun	MD	207 MD
14-Jul	138	470
14-Jul	130	470

## TABLE 2 INFLUENT AMMONIA CONCENTRATIONS

Month	Average NH3/N,	Daily Max NH3/N,
wionth	0	
	mg/L	mg/L
14-Aug	115	1,495
14-Sep	225	1,154
14-Oct	51	575
14-Nov	39	118
14-Dec	54	348
15-Jan	58	416
15-Feb	106	992
Mean	128	315
Maximum	(225)	1,495

## TABLE 2 INFLUENT AMMONIA CONCENTRATIONS

MD signifies Missing Data

# TABLE 3ROQUETTE EFFLUENT BOD5

Month	30-day Average lbs/day
NPDES Permit Limit	5,924
Jan-14	207
Feb-14	578
Mar-14	212
Apr-14	244
May-14	2,113
Jun-14	849
Jul-14	436
Aug-14	488
Sep-14	474
Oct-14	430
Nov-14	538
Dec-14	510
Jan-15	573
Feb-15	1,227
Mar-15	262
Apr-15	652
May-15	210
Average	609
Maximum	2,113

TABLE 4		
<b>ROQUETTE EFFLUENT TSS</b>		

Month	30-day Average,
	lbs/day
NPDES Permit Limit	7,138
Jan-14	1,412
Feb-14	1,077
Mar-14	765
Apr-14	2,602
May-14	2,357
Jun-14	909
Jul-14	913
Aug-14	620
Sep-14	882
Oct-14	940
Nov-14	867
Dec-14	567
Jan-15	587
Feb-15	5,576
Mar-15	845
Apr-15	1,784
May-15	861
Average	1,386
Maximun	5,576

Month	30-day Average,	30-day Average,
	lbs/day	mg/L
Jan-14	22	0.6
Feb-14	247	8.9
Mar-14	41	1.0
Apr-14	10	0.2
May-14	57	1.2
Jun-14	97	2.3
Jul-14	138	3.0
Aug-14	155	3.3
Sep-14	225	5.1
Oct-14	51	0.9
Nov-14	39	0.9
Dec-14	54	1.2
Jan-15	58	1.2
Feb-15	106	2.5
Mar-15	12	0.3
Apr-15	86	1.9
May-15	26	0.6
		2
Average	87	(2.2)
Maximum	247	8.9

#### TABLE 5 ROQUETTE EFFLUENT AMMONIA AS N

Month	Average, mg/L
Jan-14	2.2
Feb-14	2.9
Mar-14	2.7
Apr-14	15.0
May-14	3.7
Jun-14	9.0
Jul-14	2.8
Aug-14	0.7
Sep-14	-
Oct-14	2.8
Nov-14	
Dec-14	0.4
Jan-15	-
Feb-15	4.7
Mar-15	< 0.10
Apr-15	1.0
May-15	-

TABLE 6			
ROQUETTE	EFFLUENT	NITRATE	as N, mg/l

Average	4.0
Maximum	15.0