

**River Restoration Toolbox  
Glossary of Equations and  
Computations**



Iowa Department of Natural  
Resources

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## 1.0 Channel Geometry Calculations

### 1.1 Entrenchment Ratio (ER)

$$ER = \frac{W_{FPA}}{W_{BKF}}$$

where...  $W_{FPA}$  = Floodprone Width

$W_{BKF}$  = Bankfull Width

### 1.2 Bank Height Ratio (BHR)

$$BHR = \frac{LBH}{d_{max}}$$

where...  $LBH$  = Low Bank Height

$d_{max}$  = Maximum bankfull depth measured at same location as LBH

### 1.3 Mean Depth ( $d_{bkf}$ )

$$d_{bkf} = \frac{A_{BKF}}{W_{BKF}}$$

where...  $A_{BKF}$  = Bankfull Cross-Sectional Area

### 1.4 Width to Depth Ratio (W/D)

$$W/D = \frac{W_{BKF}}{d_{BKF}}$$

### 1.5 Bankfull Cross Sectional Area ( $A_{BKF}$ )

$$A_{BKF} = W_{BKF} * d_{BKF}$$

### 1.6 Channel Sinuosity (k)

$$k = \frac{SL}{VL}$$

where...  $SL$  = Stream Length

$VL$  = Valley Length

### 1.7 Meander Width Ratio (MWR)

$$MWR = \frac{W_{BLT}}{W_{BKF}}$$

where...  $W_{BLT}$  = Stream Belt Width

### 1.8 Regional Curve for Cross Sectional Area (Western Minnesota)

$$A_{BKF} = 4.7456x^{0.6102}$$

where...  $x$  = Drainage Area (sq mi)

### 1.9 Regional Curve for Cross-Sectional Area (Eastern Minnesota)

$$A_{BKF} = 5.3096x^{0.7054}$$

## 2.0 Hydraulic Calculations

### 2.1 Continuity Equation, Discharge (Q)

$$Q = VA$$

where...  $V$  = Average velocity (ft/s)

$A$  = Cross-sectional area (sq ft)

### 2.2 Velocity, Manning's (v)

$$V = \frac{1.49R^{\frac{2}{3}}S^{\frac{1}{2}}}{n}$$

where...  $R$  = Hydraulic radius (ft)

$n$  = Manning's n

$S$  = Water surface slope (ft/ft)

### 2.3 Velocity, Darcy-Weisbach (v)

$$V = \sqrt{\frac{8gRS}{f}}$$

where...  $g$  = Acceleration due to gravity

$f$  = Darcy-Weisbach friction factor

### 2.4 Velocity, Chezy (v)

$$V = c\sqrt{RS}$$

where...  $c$  = Chezy coefficient

### 2.5 Velocity, Relative Roughness (v)

$$V = \left[ 2.83 + 5.66 \log \left( \frac{R}{D_{84}} \right) \right] u^*$$

where...  $D_{84}$  = Diameter of the 84<sup>th</sup> percentile of riffle bed material

$u^*$  = Shear velocity  $(gRS)^{\frac{1}{2}}$

### 2.6 Weir Flow (Q)

$$Q = CLH^{\frac{3}{2}}$$

where...  $C$  = Weir discharge coefficient  
 $L$  = Weir length (ft)  
 $H$  = Approach head (ft)  
 $Q$  = Weir flow (cfs)

## 2.7 Shear Stress ( $\tau$ )

$$\tau = \gamma RS$$

where...  $\tau$  = Shear stress (lb/sq ft)  
 $\gamma$  = Unit weight of water  
 $R$  = Hydraulic radius (ft)  
 $S$  = Slope (ft/ft)

## 2.8 Stream Power ( $\Omega$ )

$$\Omega = \gamma QS$$

where...  $\Omega$  = Stream power (lb/s)  
 $S$  = Water surface slope (ft/ft)  
 $Q$  = Discharge (cfs)

## 2.9 Unit Stream Power ( $\omega$ )

$$\omega = \left( \frac{\Omega}{w} \right)$$

where...  $\omega$  = Unit stream power (lb/ft/s)  
 $w$  = Stream width (ft)

## 3.0 Moveable Particle Calculations

### 3.1 Critical Dimensionless Shear Stress, Andrews, Sub-Surface ( $\tau_c^*$ )

$$\tau_c^* = 0.0834 * \left( \frac{D_{50}}{D_{50}^n} \right)^{-0.872}$$

where...  $\tau_c$  = Critical dimensionless shear stress  
 $D_{50}$  = Median size of particle of riffle armor bed surface (mm)  
 $D_{50}^n$  = Median size of sub-surface particle (mm)

Note: This equation applies when  $\frac{D_{50}}{D_{50}^n}$  is between 3 and 7

### 3.2 Critical Dimensionless Shear Stress, Andrews, Bed Surface ( $\tau_c^*$ )

$$\tau_c^* = 0.0384 * \left( \frac{D_{max}}{D_{50}} \right)^{-0.887}$$

where...  $D_{max}$  = Largest particle of material representative of subsurface (mm)

$D_{50}$  = Median size particle of the bed surface (mm)

Note: This equation applies when  $\frac{D_{max}}{D_{50}}$  is between 1.3 and 3

### 3.3 Required Depth to Entrain Largest Particle (Competency)

$$d = \frac{\tau_c^* \gamma_s D_{max}}{S}$$

where...  $d$  = Depth required to entrain largest particle (ft)

$D_{max}$  = Largest particle of material representative of subsurface (mm)

$\tau_c^*$  = Critical dimensionless shear stress

$\gamma_s$  = Submerged specific weight of sediment

$S$  = Slope (ft/ft)

### 3.4 Required Slope to Entrain Largest Particle (Competency)

$$S = \frac{\tau_c^* \gamma_s D_{max}}{d}$$

### 3.5 Lanes's Tractive Force (Shear stress)

$$\tau_c = dS = D$$

where...  $\tau_c$  = Shear stress (kg/sq m)

$d$  = Mean depth (mm)

$S$  = Water surface slope

$D$  = Incipient particle diameter (cm)

## 4.0 Scour Calculations

Refer to NRCS Technical Supplement 14B – Scour Calculations (Aug 2007) for more information

### 4.1 Total Scour Depth ( $z_t$ )

$$z_t = FS[Z_{ad} + Z_b + Z_{bf} + Z_s]$$

where...	$z_t$	=	Total scour depth
	$FS$	=	Factor of safety
	$Z_{ad}$	=	Bed elevation changes due to reach-scale deposition (aggradation) or bed erosion (degradation)
	$Z_c$	=	Concentration scour
	$Z_b$	=	Scour on outside of bend
	$Z_{bf}$	=	Bedform trough depth
	$Z_s$	=	Local scour depth associated with a structure

Refer to NRCS Technical Supplement 14B for guidance on how to compute each component of scour.

## 5.0 Permissible Shear and Velocity for Selected Lining Materials

Refer to Stability Thresholds for Stream Restoration Materials by Craig Fischenich (May 2001) for more information. Table 2 within this document is provided below.

Boundary Category	Boundary Type	Permissible Shear Stress (lb/sq ft)	Permissible Velocity (ft/sec)	Citation(s)
<u>Soils</u>	Fine colloidal sand	0.02 - 0.03	1.5	A
	Sandy loam (noncolloidal)	0.03 - 0.04	1.75	A
	Alluvial silt (noncolloidal)	0.045 - 0.05	2	A
	Silty loam (noncolloidal)	0.045 - 0.05	1.75 - 2.25	A
	Firm loam	0.075	2.5	A
	Fine gravels	0.075	2.5	A
	Stiff clay	0.26	3 - 4.5	A, F
	Alluvial silt (colloidal)	0.26	3.75	A
	Graded loam to cobbles	0.38	3.75	A
	Graded silts to cobbles	0.43	4	A
	Shales and hardpan	0.67	6	A
	<u>Gravel/Cobble</u>	1-in.	0.33	2.5 - 5
2-in.		0.67	3 - 6	A
6-in.		2.0	4 - 7.5	A
12-in.		4.0	5.5 - 12	A
<u>Vegetation</u>	Class A turf	3.7	6 - 8	E, N
	Class B turf	2.1	4 - 7	E, N
	Class C turf	1.0	3.5	E, N
	Long native grasses	1.2 - 1.7	4 - 6	G, H, L, N
	Short native and bunch grass	0.7 - 0.95	3 - 4	G, H, L, N
	Reed plantings	0.1-0.6	N/A	E, N
	Hardwood tree plantings	0.41-2.5	N/A	E, N
<u>Temporary Degradable RECPs</u>	Jute net	0.45	1 - 2.5	E, H, M
	Straw with net	1.5 - 1.65	1 - 3	E, H, M
	Coconut fiber with net	2.25	3 - 4	E, M
	Fiberglass roving	2.00	2.5 - 7	E, H, M
<u>Non-Degradable RECPs</u>	Unvegetated	3.00	5 - 7	E, G, M
	Partially established	4.0-6.0	7.5 - 15	E, G, M
	Fully vegetated	8.00	8 - 21	F, L, M
<u>Riprap</u>	6 - in. $d_{50}$	2.5	5 - 10	H
	9 - in. $d_{50}$	3.8	7 - 11	H
	12 - in. $d_{50}$	5.1	10 - 13	H
	18 - in. $d_{50}$	7.6	12 - 16	H
	24 - in. $d_{50}$	10.1	14 - 18	E
<u>Soil Bioengineering</u>	Wattles	0.2 - 1.0	3	C, I, J, N
	Reed fascine	0.6-1.25	5	E
	Coir roll	3 - 5	8	E, M, N
	Vegetated coir mat	4 - 8	9.5	E, M, N
	Live brush mattress (initial)	0.4 - 4.1	4	B, E, I
	Live brush mattress (grown)	3.90-8.2	12	B, C, E, I, N
	Brush layering (initial/grown)	0.4 - 6.25	12	E, I, N
	Live fascine	1.25-3.10	6 - 8	C, E, I, J
	Live willow stakes	2.10-3.10	3 - 10	E, N, O
<u>Hard Surfacing</u>	Gabions	10	14 - 19	D
	Concrete	12.5	>18	H

## 6.0 Rock Sizing Equations

Refer to US Army Corps of Engineers Engineering Manual EM 1110-2-1601 (July 1991) and revised (June 1994) for more information on rock sizing.

### 6.1 Rock Sizing, $D_{30}$

$$D_{30} = \frac{1.95(S^{0.55})(q^{0.667})}{g^{0.333}}$$

where...  $D_{30}$  = Size fraction of which 30% of particles are finer

$S$  = Slope (ft/ft)

$q$  = Unit discharge (Q/bottom width) (cfs/ft)

$g$  = Gravitational constant

### 6.2 Rock Sizing, $D_{50}$

$$D_{50} = D_{30} * 1.22$$

where...  $D_{50}$  = Size fraction of which 50% of particles are finer

### 6.3 Minimum Rock Thickness ( $R_T$ )

$$R_T = D_{100} * 1.5$$

where...  $D_{100}$  = Largest size fraction of rock present in gradation

## 7.0 Buoyancy Calculation

### 7.1 Buoyancy ( $F_b$ )

$$F_b = \rho g V$$

where...  $g$  = Gravitational constant

$\rho$  = Density of the liquid (lb/cu ft)

$V$  = Volume of liquid displaced (cu ft)

### 7.2 Buoyancy ( $F_b$ )

$$F_b = \rho g h A$$

where...  $h$  = Height of water displaced by a floating object (ft)

$A$  = Surface area of floating object (sq ft)